

STRUCTURAL NOTES

A. BASIS OF DESIGN:

1. THE STRUCTURAL DESIGN HAS BEEN PERFORMED IN ACCORDANCE WITH THE 2022 CALIFORNIA BUILDING CODE (CBC).
2. STRUCTURAL MATERIALS INDICATED SHALL COMPLY WITH THE 2022 CALIFORNIA GREEN BUILDING STANDARDS CODE (CGBC), INCLUDING 2024 SUPPLEMENT SECTIONS 5.105 OR 5.409.
3. LIVE LOADS (MAY BE REDUCED IN ACCORDANCE WITH THE BUILDING CODE)

ROOF	20 psf
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4. SEISMIC DESIGN DATA

RISK CATEGORY	II
SEISMIC IMPORTANCE FACTOR	$I_e = 1.0$
MAPPED SPECTRAL ACCELERATION	$S_{DS} = 0.895$
MAPPED SPECTRAL ACCELERATION	$S_1 = 0.315$
SITE CLASS	D [DEFAULT]
SITE COEFFICIENT	$F_a = 1.2$
SITE COEFFICIENT	$F_v = N/A$
DESIGN SPECTRAL ACCELERATION	$S_{DS} = 0.718$
DESIGN SPECTRAL ACCELERATION	$S_{D1} = N/A$
SEISMIC DESIGN CATEGORY	D
PARTITION LOADING AT ROOFS	5 psf
ANALYSIS PROCEDURE USED	EQUIV LATERAL FORCE
FOR LONGITUDINAL DIRECTION	
SEISMIC FORCE RESISTING SYSTEM	
RESPONSE MODIFICATION FACTOR	$R =$
SYSTEM OVERSTRENGTH FACTOR	$\Omega =$
DEFLECTION AMPLIFICATION FACTOR	$C_d =$
REDUNDANCY FACTOR	$p = 1.0$
SEISMIC RESPONSE COEFFICIENT	$C_s =$
DESIGN BASE SHEAR	$V =$
FOR TRANSVERSE DIRECTION	
SEISMIC FORCE RESISTING SYSTEM	
RESPONSE MODIFICATION FACTOR	$R =$
SYSTEM OVERSTRENGTH FACTOR	$\Omega =$
DEFLECTION AMPLIFICATION FACTOR	$C_d =$
REDUNDANCY FACTOR	$p = 1.0$
SEISMIC RESPONSE COEFFICIENT	$C_s =$
DESIGN BASE SHEAR	$V =$

5. COMPONENT SEISMIC DESIGN DATA

COMPONENT	$R_o$	$R_d$	$\Omega_o$	$I_o$
Walk-In Cooler/Freezer				

6. WIND DESIGN DATA

RISK CATEGORY	III
BASIC WIND SPEED	$V = 103$ mph
EXPOSURE CATEGORY	C (B)
NORTH QUADRANT	C (B)
EAST QUADRANT	C (B)
SOUTH QUADRANT	C (B)
WEST QUADRANT	C (B)
ENCLOSURE CATEGORY	ENCLOSED
GUST & INTERNAL PRESSURE COEFF	$GCF_i = 0.18$
DIRECTIONALITY FACTOR	$K_{zt} = 0.85$
GROUND ELEVATION FACTOR	$K_{g1} = 1.00$
TOPOGRAPHIC FACTOR	$K_{zt} = 0.00$
TOPOGRAPHIC FACTOR	$K_1 = 0.00$
TOPOGRAPHIC FACTOR	$K_2 = 0.00$
TOPOGRAPHIC FACTOR	$K_3 = 0.00$

B. SUBMITTALS:

1. THE CONTRACTOR SHALL MAKE SUBMITTALS PRIOR TO FABRICATION AS REQUIRED BY THE WRITTEN SPECIFICATIONS AND SHALL INCLUDE AS A MINIMUM THE FOLLOWING SUBMITTALS:
- A. CONCRETE MIX DESIGNS.
- B. REINFORCING STEEL DRAWINGS.
- C. WELDED DEFORMED WIRE REINFORCING DRAWINGS, WITH CODE-BASED DEMONSTRATION OF EQUIVALENCY TO SPECIFIED BARS.
- D. STRUCTURAL STEEL DRAWINGS
- E. COLD-FORMED STEEL FRAMING DRAWINGS.
- F. WELDING PROCEDURES (SHOP AND FIELD WELDING).
2. THE FOLLOWING SUBMITTALS ARE NOT REQUIRED FOR STRUCTURAL REVIEW:
- A. SHORING AND BRACING.
- B. PICK-UP INSERTS.
- C. UNSPLICED REBAR AT SLAB-ON-GRADE AND FOOTINGS
- D. FORMWORK.
- E. STRUCTURAL STEEL MILL REPORTS.
- F. WELDER CERTIFICATIONS
3. STEEL REINFORCING LISTS AND QUANTITIES AND LENGTHS OF ALL MATERIALS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO ASSURE COMPLIANCE WITH THE PLANS. ENGINEER WILL NOT REVIEW.
4. SUBMITTALS MADE TO THE ENGINEER FOR REVIEW SHALL BE STAMPED AND SIGNED BY THE CONTRACTOR INDICATING THE CONTRACTORS PRIOR REVIEW AND THAT THE SUBMITTAL IS IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
5. SUBMITTALS SHALL BE MADE IN ELECTRONIC (PDF) FORMAT. SUBMITTALS WILL BE PROCESSED AND RETURNED ELECTRONICALLY.

C. GENERAL:

1. SPECIFIC NOTES AND DETAILS ON THE STRUCTURAL DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS.
2. THE REQUIREMENTS ON THE STRUCTURAL DRAWINGS SHALL TAKE PRECEDENCE OVER THE STRUCTURAL BUILDING INFORMATION MODEL. THE STRUCTURAL BUILDING INFORMATION MODEL SHALL NOT BE RELIED ON FOR UNDERSTANDING CONSTRUCTION REQUIREMENTS.
3. WHERE NO DETAILS ARE SHOWN, OR NOTED IN ANY PART OF THE WORK THE DETAILS FOR OTHER SIMILAR WORK SHALL APPLY
4. DETAILS IDENTIFIED AS TYPICAL, SHALL APPLY IN ESTIMATING AND CONSTRUCTION TO EVERY LIKE CONDITION WHETHER OR NOT THE REFERENCE IS REPEATED.
5. THE STRUCTURAL DRAWINGS AND STRUCTURAL BUILDING INFORMATION MODEL SHALL NOT BE SCALED. COORDINATE DIMENSION, ELEVATION, SLOPE AND DRAINAGE REQUIREMENTS WITH THE ARCHITECTURAL DRAWINGS.
6. STANDARDS REFERENCED ON THE STRUCTURAL DRAWINGS REFER TO THE PERTINENT EDITION UNDER THE APPLICABLE BUILDING CODE.
7. THE RESPONSIBILITY FOR THE REVIEW AND COORDINATION OF DRAWINGS AND SPECIFICATIONS PRIOR TO THE START OF RELATED CONSTRUCTION SHALL BEAR ON THE CONTRACTOR. DISCREPANCIES THAT EXIST SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER IN A TIMELY MANNER, PRIOR TO START OF RELATED CONSTRUCTION.
8. WORK PERFORMED IN CONFLICT WITH THE STRUCTURAL DRAWINGS OR APPLICABLE BUILDING CODE REQUIREMENTS SHALL BE CORRECTED AT THE EXPENSE OF THE CONTRACTOR.
9. EXISTING CONDITIONS SHALL BE VERIFIED BEFORE STARTING RELATED WORK. EXISTING CONDITIONS THAT ARE NOT REFLECTED ON THE STRUCTURAL DRAWINGS OR THAT DEViate FROM THE MAXIMUM OR MINIMUM DIMENSIONS INDICATED SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER IN A TIMELY MANNER. SUCH CONDITIONS MAY INCLUDE CONFLICT IN GRADES, ADVERSE SOIL CONDITIONS, PRESENCE OF GROUND WATER, UNCOVERED OR UNEXPECTED EXISTING CONSTRUCTION CONFIGURATIONS, ETC.
10. MATERIALS AND WORKMANSHIP SHALL CONFORM TO REQUIREMENTS OF APPLICABLE REGULATIONS AND THE BUILDING CODE AS AMENDED AND ADOPTED BY THE BUILDING OFFICIAL.
11. LOADS TO THE BUILDING EXCEEDING THE LOADS INDICATED ON THE PLANS, OR ANY LOADS EXCEEDING 400 POUNDS THAT ARE NOT INDICATED ON THE STRUCTURAL DRAWINGS SHALL BE REPORTED TO THE ENGINEER.

D. TEMPORARY WORK AND SITE SAFETY:

1. THE STRUCTURAL DRAWINGS SHOW THE REQUIREMENTS FOR THE COMPLETED STRUCTURE ONLY. TEMPORARY WORKS REQUIRED TO COMPLETE THE CONSTRUCTION PROCESS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE STRUCTURAL ENGINEER SHALL NOT BE RESPONSIBLE FOR THE DESIGN OR FIELD VERIFICATION OF TEMPORARY AND ANCILLARY WORK.
2. THE RESPONSIBILITY FOR SAFETY IN AND AROUND THE JOBSITE SHALL BEAR ON THE CONTRACTOR. PROPER AND SAFE METHODS OF CONSTRUCTION SHALL BE EMPLOYED AT ALL TIMES INCLUDING THE STABILIZING OF INCOMPLETE STRUCTURES, FORMWORK, SHORING, RESHORING, FALSEWORK, PLATFORMS, SCAFFOLDING, BARRIERS, WALKWAYS, ETC. AND INCLUDING CONTROL OF THE INTENSITY, DURATION AND LOCATION OF CONSTRUCTION LOADS.
3. THE RESPONSIBILITY FOR THE DESIGN AND INSTALLATION OF ALL CRIBBING, SHEATHING, UNDERPINNING, AND SHORING REQUIRED TO SAFELY RETAIN ALL GRADES AND STRUCTURES SHALL BEAR ON THE CONTRACTOR.
4. CONSTRUCTION MATERIALS SHALL BE SPREAD OUT IF PLACED ON A STRUCTURE. LOADS SHALL NOT EXCEED THE DESIGN LIVE LOAD INDICATED. WHERE THE STRUCTURE HAS NOT ATTAINED FINAL DESIGN STRENGTH, ADEQUATE SHORING AND OR BRACING SHALL BE INSTALLED.

E. DEMOLITION:

1. THE RESPONSIBILITY FOR NECESSARY SHORING OR BRACING OF THE EXISTING STRUCTURE DURING DEMOLITION PROCEDURES SHALL BEAR ON THE CONTRACTOR.
2. DEVIATIONS FROM EXISTING CONDITIONS AS INDICATED ON THE STRUCTURAL DRAWINGS SHALL BE RESOLVED WITH THE ENGINEER PRIOR TO PROCEEDING WITH DEMOLITION WORK.
3. RESPONSIBILITY FOR COORDINATION OF DEMOLITION WORK WITH THE BUILDING OFFICIAL AND OTHER GOVERNING AUTHORITIES SHALL BEAR ON THE CONTRACTOR. EXITS SHALL BE MAINTAINED AS REQUIRED FOR SAFE LEGAL OPERATION OF THE FACILITY.
4. ELEMENTS THAT WILL NOT BE DEMOLISHED SHALL BE PROTECTED FROM DAMAGE.
5. SAW CUT LINES SHALL BE TRUE AND NEAT. CORNERS SHALL NOT BE OVER CUT.
6. THESE STRUCTURAL DRAWINGS DO NOT INDICATE THE PHASING OF DEMOLITION. THE RESPONSIBILITY FOR THE SCHEDULING AND COORDINATION OF THE WORK SHALL BEAR ON THE CONTRACTOR. WORK SHALL BE COORDINATED TO LEAST IMPACT THE OPERATION OF THE EXISTING FACILITY.

F. FOUNDATION:

1. AS A CALIFORNIA-LICENSED ENGINEER, THE ENGINEER OF RECORD FOR THE STRUCTURAL DESIGN HAS CLASSIFIED THE UNDISTURBED, NATIVE SOILS TO BE \_\_\_\_\_ IN ACCORDANCE WITH TABLE 1808.2 OF THE BUILDING CODE. AN ALLOWABLE FOUNDATION BEARING PRESSURE OF [1,500] [2,000] PSF HAS BEEN ASSIGNED FOR THE DESIGN OF FOUNDATIONS RELATED TO THIS PROJECT.

2. IF THE BUILDING OFFICIAL SUSPECTS FILL MATERIAL, EXPANSIVE SOIL OR GEOLOGIC INSTABILITY UPON OBSERVATION OF THE FOUNDATION EXCAVATIONS, A GEOLOGICAL INVESTIGATION REPORT AND CONSTRUCTION DRAWINGS THAT ARE COMPLIANT WITH THE RECOMMENDATIONS OF THAT GEOLOGICAL INVESTIGATION REPORT MAY BE REQUIRED TO BE SUBMITTED FOR REVIEW BY THE BUILDING OFFICIAL PRIOR TO CONSTRUCTION OF THE FOUNDATIONS.
3. SPECIAL INSPECTION AND TESTING IS REQUIRED IN ACCORDANCE WITH SECTIONS 1704 AND 1705 OF THE BUILDING CODE AND THE 'STATEMENT OF SPECIAL INSPECTIONS' ON THESE CONSTRUCTION DOCUMENTS.
4. THE MAXIMUM ALLOWABLE SOIL BEARING PRESSURE SHALL BE \_\_\_\_\_ psf. ALLOWABLE BEARING MAY BE INCREASED BY \_\_\_\_\_ psf AND \_\_\_\_\_ psf FOR EACH ADDITIONAL FOOT OF FOUNDATION WIDTH AND DEPTH, RESPECTIVELY, TO A MAXIMUM OF \_\_\_\_\_ psf. THE RESULTING ALLOWABLE BEARING VALUE MAY BE INCREASED BY 1/3 FOR WIND AND SEISMIC LOAD CASES.
5. THE EXPANSION INDEX HAS BEEN DETERMINED TO BE GREATER THAN 20 AND THE RECOMMENDATIONS OF THE GEOTECHNICAL REPORT HAVE BEEN INCORPORATED INTO THESE DRAWINGS.
6. THE EXPANSION INDEX HAS BEEN DETERMINED TO BE 20 OR LESS AND NO SPECIAL DESIGN RECOMMENDATIONS ARE REQUIRED.
7. FOOTING AND UTILITY TRENCH BACKFILL SHALL BE MECHANICALLY COMPACTED IN LAYERS SUBJECT TO THE APPROVAL OF THE GEOTECHNICAL ENGINEER. FLOODING WILL NOT BE PERMITTED.
8. LOOSE SOIL AND FILL MATERIAL SHALL BE COMPACTED ACCORDING TO THE REQUIREMENTS OF THE SOILS REPORT.
9. COMPACTION TEST REPORTS FOR FILL BY A QUALIFIED TESTING LAB SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER, GEOTECHNICAL ENGINEER AND BUILDING OFFICIAL PRIOR TO REQUESTING FOUNDATION INSPECTION.
10. FOOTING DEPTHS INDICATED ON THE STRUCTURAL DRAWINGS ARE BELIEVED TO BE IN SUITABLE BEARING MATERIALS AND ARE INDICATED FOR COST ESTIMATING PURPOSES ONLY. THE GEOTECHNICAL ENGINEER MAY REQUIRE FOUNDATION DEPTHS TO BE INCREASED. THE RESPONSIBILITY FOR CONFORMING TO THE GEOTECHNICAL REPORT RECOMMENDATIONS REGARDING DEPTH OF FOOTINGS SHALL BEAR ON THE CONTRACTOR.
11. FOOTING ELEVATIONS SHALL BE LOCATED SUCH THAT THEIR BEARING IS A MINIMUM HORIZONTAL DISTANCE OF \_\_\_\_\_ FROM THE DAYLIGHT OF AN ADJACENT SLOPE OR AS RECOMMENDED WITHIN THE GEOTECHNICAL REPORT.
12. ANCHOR BOLTS, DOWELS AND HOLD-DOWN ANCHORS SHALL BE TIED IN PLACE PRIOR TO FOUNDATION INSPECTION.

G. WELDING:

1. WELDING OF STRUCTURAL STEEL SHALL BE PERFORMED BY CERTIFIED WELDERS IN ACCORDANCE WITH THE PROVISIONS OF THE AMERICAN WELDING SOCIETY (AWS) D1.1. ELECTRODE FILLER MATERIAL SHALL BE A MINIMUM OF E70XX U.O.N
2. WELDING OF COLD-FORMED STEEL DECK AND COLD-FORMED FRAMING SHALL BE PERFORMED BY WELDERS CERTIFIED FOR SHEET STEEL IN ACCORDANCE WITH THE PROVISIONS OF THE AMERICAN WELDING SOCIETY (AWS) D1.3. E60XX ELECTRODES SHALL BE USED FOR WELDING OF COLD-FORMED STEEL DECK AND COLD-FORMED FRAMING.
3. WELDING OF REINFORCING BARS SHALL BE PERFORMED BY WELDERS CERTIFIED FOR REINFORCING BARS IN ACCORDANCE WITH THE PROVISIONS OF THE AMERICAN WELDING SOCIETY (AWS) D1.4. E60XX ELECTRODES SHALL BE USED FOR WELDING OF REINFORCING BAR TO REINFORCING BAR. E70XX ELECTRODES SHALL BE USED FOR WELDING OF REINFORCING BAR TO ROLLED STEEL SHAPES AND PLATES.
4. SPECIAL INSPECTION AND TESTING IS REQUIRED IN ACCORDANCE WITH SECTIONS 1704 AND 1705 OF THE BUILDING CODE AND THE 'STATEMENT OF SPECIAL INSPECTIONS' ON THESE CONSTRUCTION DOCUMENTS.
5. WELDING ELECTRODES FOR THE SHIELDED METAL-ARC WELDING (S.M.A.W.) PROCESS AND WELDING ELECTRODES SHALL CONFORM TO AWS A5.1 'SPECIFICATION FOR CARBON STEEL ELECTRODES FOR SHIELDED METAL ARC WELDING.'
6. WELDING ELECTRODES FOR THE FLUX CORED ARC WELDING (F.C.A.W.) PROCESS AND WELDING ELECTRODES SHALL CONFORM TO AWS A5.20 'SPECIFICATION FOR CARBON STEEL ELECTRODES FOR FLUX CORED ARC WELDING.'
7. WELDS SHALL HAVE A WELD CONTROLLED SEQUENCE AND TECHNIQUE IN ORDER TO MINIMIZE SHRINKAGE STRESSES AND DISTORTION.
8. WELDED CONNECTIONS OF COLLECTORS ARE CONSIDERED TO BE PART OF THE SEISMIC FORCE RESISTING SYSTEM AND SHALL CONFORM TO THE REQUIREMENTS OF AISC 341. FILLER MATERIAL FOR THESE WELDS SHALL MEET AWS D1.8 REQUIREMENTS FOR H18 WELDS.

H. REINFORCING STEEL:

1. DETAILING, FABRICATION AND ERECTION OF REINFORCING BARS SHALL BE PERFORMED IN ACCORDANCE WITH ACI 318R, 'GUIDE TO PRESENTING REINFORCING STEEL DESIGN DETAILS.'
2. SPECIAL INSPECTION AND TESTING IS REQUIRED IN ACCORDANCE WITH SECTIONS 1704 AND 1705 OF THE BUILDING CODE AND THE 'STATEMENT OF SPECIAL INSPECTIONS' ON THESE CONSTRUCTION DOCUMENTS.
3. REINFORCING BARS SHALL CONFORM TO A MAXIMUM UNFABRICATED GLOBAL WARMING POTENTIAL (GWP) LIMIT OF 1.58 MT CO<sub>2</sub>e/g / MT.
4. REINFORCING BARS SHALL CONFORM TO ASTM A 615, GRADE 60, U.O.N. ADDITIONALLY, ASTM A 615 REINFORCING BARS SHALL ALSO COMFORM TO THE FOLLOWING:
- |  |            |
|--|------------|
|  | GRADE 60   |
| MINIMUM ACTUAL TENSILE STRENGTH                          | 80,000 PSI |
| MINIMUM ACTUAL YIELD STRENGTH                            | 60,000 PSI |
| ACTUAL TENSILE STRENGTH DIVIDED BY ACTUAL YIELD STRENGTH | 1.10 (MIN) |
5. WELDED REINFORCING BARS SHALL CONFORM TO ASTM A 706. BARS TO BE WELDED CONFORMING TO ASTM A 615, GRADE 60 MAY BE SUBSTITUTED FOR BARS CONFORMING TO ASTM A 706, GRADE 60, PROVIDED THAT THE WELDING OF BARS COMPLIES WITH AWS D1.4 AND THE MINIMUM SPECIFICATIONS FOR WELDING OF REINFORCING STEEL INCLUDED HEREIN.
6. WELDED DEFORMED WIRE REINFORCEMENT SHALL CONFORM TO ASTM A 1064, USING DEFORMED WIRE CONFORMING TO ASTM A 1064, GRADE 60. WELDED DEFORMED WIRE REINFORCEMENT MAY BE SUBSTITUTED FOR REINFORCING BARS IN THE FOLLOWING LOCATIONS PROVIDED THAT IT IS QUANTITATIVELY DEMONSTRATED TO PROVIDE EQUIVALENT STRENGTH AS THE SPECIFIED REINFORCING BARS.
- A. SLABS-ON-GRADE
- B. FLEXURAL REINFORCEMENT IN 1-WAY SLABS.
- C. SHEAR REINFORCEMENT IN BEAMS THAT DO NOT DIRECTLY SUPPORT OTHER BEAMS.
7. WELDED REINFORCEMENT GRIDS SHALL BE BAUGRID (ICC #ESR-2352). ALTERNATE PRODUCTS SHALL CARRY AN EVALUATION APPROVAL AND SHALL BE APPROVED BY THE ENGINEER IN WRITING PRIOR TO DELIVERY TO THE JOBSITE.
8. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A 185. MINIMUM LAP OF WWF SHALL BE ONE FULL MESH + 2'.
9. REINFORCING BAR LAP SPLICES SHALL BE: CLASS B. (18" MIN.) FOR CONCRETE, U.O.N. 65 BAR DIA. (24" MIN.) FOR MASONRY, U.O.N.
10. DETAILS OF REINFORCEMENT SHALL COMPLY WITH THE PROVISIONS OF ACI 318.
11. WHERE HOOKS ARE ILLUSTRATED AS 90-DEGREE HOOKS, 180-DEGREE HOOKS MAY BE USED IN LIEU OF 90-DEGREE HOOKS.
12. WHERE CONVENTIONAL STIRRUPS, TIES, HOOPS OR CROSS-TIES ARE ILLUSTRATED, CONTINUOUSLY-WOUND, MACHINE-PREASSEMBLED, TRANSVERSE REINFORCEMENT MAY BE SUBSTITUTED, PROVIDED THAT:
- A. THE RESULTING CONFIGURATION PROVIDES FOR PROPER CONSOLIDATION OF CONCRETE.
- B. ASTM A 706 REINFORCING IS USED FOR TRANSVERSE REINFORCING, AND
- C. WHERE FUSION-WELDED HOLDING WIRES ARE USED, THEY SHALL BE ¼ INCH DIAMETER, CONFORMING TO ASTM A 82 OR A 486.
13. REINFORCING BARS FOR CONCRETE SHALL BE PROVIDED WITH THE FOLLOWING MINIMUM COVER:
- |   |        |
|---|--------|
| CONC. CAST AGAINST EARTH                | 3"     |
| FORMED CONC. EXPOSED TO EARTH / WEATHER |        |
| NO 5 OR SMALLER                         | 1-1/2" |
| NO 6 OR LARGER                          | 2"     |
| SLABS (#1 AND SMALLER)                  | 3/4"   |
14. #3 SPACER TIES SHALL BE INSTALLED AT 30" ON CENTER IN BEAMS AND FOOTINGS TO SECURE REINFORCING BARS IN PLACE, U.O.N.
15. REINFORCEMENT SUPPORTS SHALL BE MANUFACTURED OF NONCORROSIVE MATERIAL
16. AN ALLOWANCE OF \_\_\_\_\_ POUNDS OF [GRADE 60] [GRADE 60] REINFORCEMENT SHALL BE PROVIDED FOR PLACEMENT IN STRUCTURAL SLABS AS DIRECTED BY THE ENGINEER DURING FIELD OBSERVATION.

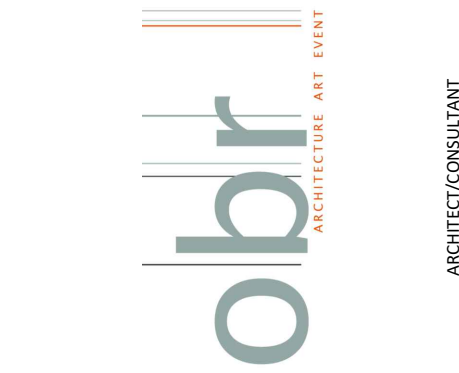
I. MINIMUM SPECIFICATIONS FOR WELDING OF REINFORCING STEEL:

1. PRIOR TO WELDING, THE CHEMICAL ANALYSIS AND THE CARBON EQUIVALENT OF STEEL (C.E.) SHALL BE DETERMINED. THE CARBON EQUIVALENT OF REINFORCING STEEL OR SPLICE MATERIAL SHALL BE CALCULATED FROM ITS CHEMICAL COMPOSITION AND SHALL BE LESS THAN 0.55.
2. MILL REPORTS STATING THE CHEMICAL COMPOSITION AND THE CARBON EQUIVALENT DETERMINED BY HEAT ANALYSIS BY THE MANUFACTURER SHALL BE FURNISHED FOR REVIEW BY THE REGISTERED SPECIAL INSPECTOR. STEEL DELIVERED TO THE JOB SITE SHALL BE CLEARLY IDENTIFIED SO AGREEMENT WITH THE HEAT NUMBER(S) OF THE MILL REPORT(S) CAN BE VERIFIED.
3. JOINT WELDING PROCEDURES FOR REINFORCING BAR SPLICES, BAR CONNECTIONS, AND INSERT CONNECTIONS THAT ARE TO BE EMPLOYED SHALL BE ESTABLISHED IN A PROCEDURE SPECIFICATION AND SHALL BE QUALIFIED PRIOR TO USE BY TESTS AS PRESCRIBED IN AWS D1.4:
- A. WELDING PROCEDURES FOR FILLET WELDS WHICH CONFORM TO THE APPLICABLE PROVISIONS OF AWS D1.4 SHALL BE DEEMED AS PREQUALIFIED
- B. THE BUILDING OFFICIAL MAY ACCEPT EVIDENCE OF PREVIOUS QUALIFICATION OF THE JOINT WELDING PROCEDURE.
4. PROCEDURE QUALIFICATION TESTS SHALL BE WITNESSED AND TESTED BY AN APPROVED TESTING AGENCY.
5. THE RESULTS OF THE PROCEDURE QUALIFICATION TESTS AND/OR THE PROCEDURE SPECIFICATION SHALL BE SUBMITTED TO AND APPROVED BY THE DESIGN ENGINEER AND THE BUILDING INSPECTION DEPARTMENT. APPROVED PROCEDURE SPECIFICATIONS SHALL BE AVAILABLE FOR REVIEW BY THE REGISTERED SPECIAL INSPECTOR AT THE SITE WHERE WELDING IS PERFORMED.

DSA



FOR DSA REVIEW ONLY - NOT FOR CONSTRUCTION



PREPARED FOR THE  
BOARD OF EDUCATION

PREPARED BY THE  
ob|ARCHITECTURE  
2419 EL CAJON BLVD, SAN DIEGO, CA 92104

STRUCTURAL NOTES

SAN DIEGO UNIFIED SCHOOL DISTRICT  
KITCHEN MODIFICATIONS GROUP 6  
BELL MIDDLE SCHOOL  
620 BRIARWOOD RD.  
SAN DIEGO, CA 92139  
BID SET

PROJECT NO. SDUF-002

DATE 04/12/25

REVISIONS SHEET NO.

S0.1